Bridge Deck Drain Design Procedure

DEFINITION OF SYMBOLS

A  =  contributing drainage area  (acres)
C  =  Runoff coefficient (ratio of impervious to pervious drainage area).  Use 0.9
d   =  water film depth   Use 0.006 ft.
E  =  inlet interception efficiency
    \[ E = R_f \times E_0 \]
E_o  =  Ratio of frontal flow to total gutter flow
    \[ E_o = 1 - \left[ 1 - \frac{W_d}{T} \right]^{2.67} \]
G1  =  Incoming tangent grade on a vertical curve.   Ft/ft
G2  =  Outgoing tangent grade on a vertical curve.   Ft/ft
i  =  design rainfall intensity (in/hr)   Use the value from the Rational Method and compare to the Avoidance of Hydroplaning and Driver Vision Impairment.

Rational Method
1.  Select the rainfall intensity zone from the map.
2.  Make a trial selection of duration and compute the trial intensity.
    \[ t_c = t_0 + t_g \]
    \[ t_0 = 0.93 \times \frac{(W_p \times n)^{0.6}}{(C \times i^{0.4} \times S_0^{0.3})} \]
    \[ t_g = 484 \times \frac{S_S \times T^2}{C \times i \times W_p} \]
3.  Using the trial intensity value, compute the total time of concentration
4.  The design intensity is the value when the trial duration equals the calculated total time of concentration.

Avoidance of Hydroplaning

\[ t = \left[ \frac{64904.4}{C \times n} \right] \left[ \frac{S_S}{(S_S + S_S^{0.25})} \right] \left[ \frac{d^{1.67}}{(W_p - T)} \right] \] (in/hr.)

Driver Vision Impairment
\[ i = 4.0 \text{ in/hr} \]

IDF = intensity-duration-frequency curve.  See Figure 1 for the Rainfall Intensity Zone.  The inlet design is based upon a 25-year frequency.  The intensity for a given duration can be computed from the following equations:

Zone 1

<table>
<thead>
<tr>
<th>Duration</th>
<th>Equation</th>
</tr>
</thead>
<tbody>
<tr>
<td>&lt; 60 minutes</td>
<td>[ i = 10 \times 0.894094 - 0.55912 \times \log(d) ]</td>
</tr>
<tr>
<td>&gt; 60 minutes</td>
<td>[ i = 10 \times 1.132115 - 0.69398 \times \log(d) ]</td>
</tr>
</tbody>
</table>

Zone 2

<table>
<thead>
<tr>
<th>Duration</th>
<th>Equation</th>
</tr>
</thead>
<tbody>
<tr>
<td>&lt; 60 minutes</td>
<td>[ i = 10 \times 1.132076 - 0.60121 \times \log(d) ]</td>
</tr>
<tr>
<td>&gt; 60 minutes</td>
<td>[ i = 10 \times 1.375147 - 0.75708 \times \log(d) ]</td>
</tr>
</tbody>
</table>

K  =  inlet interception coefficient on vertical curve bridges

\[ K = 1 - (1 - E) \times \left( \frac{S_u}{S} \right)^{0.5} \]

L_c  =  distance between deck drains after the first inlet (ft)

Drains on tangent grade

\[ L_c = \frac{4350 \times Q_f \times E}{C \times i \times W_p} \]

Drains on flat grade

\[ L_c = \frac{1312 \times S_S^{1.44} \times T^{2.11}}{(n \times C \times i \times W_p)^{0.67}} \]
\[ L_0 = \text{distance to first deck drain (ft)} \]
\[ L_0 = \frac{43560 \times Q_f}{C \times i \times W_p} \]
\[ L_D = \text{length of deck drain parallel to gutter flow (ft)} \]
- Type 1 Drain = 0.78 ft.
- Type 2 Drain = 0.79 ft.
\[ L_{vc} = \text{length of vertical curve (ft)} \]
\[ n = \text{Manning's friction coefficient (0.016)} \]
\[ P = \frac{\left( C \times i \times W_p \right)^{0.33} \times T^{0.61}}{102.5 \times S_x^{0.09} \times n^{0.67}} \]
\[ Q_f = \text{full gutter flow at design spread (cfs)} \]
\[ Q_f = \frac{0.56}{n} \times S_x^{0.67} \times S_{x}^{0.5} \times T^{2.67} \]
\[ Q_R = \text{flow calculated with rational formula (cfs)} \]
\[ Q_R = C \times i \times A \]
\[ R_f = \text{frontal flow interception efficiency} \]
\[ R_f = 1 - 0.09 \times (V - V_o) \]
\[ S = \text{grade of bridge deck at a given drain (ft/ft)} \]
Drains on vertical curve
\[ S = G1 + \frac{L_{vc}}{L} \]
\[ S_u = \text{longitudinal grade for upstream inlet on vertical curve bridge} \]
\[ S_c = \text{cross slope of deck (ft/ft)} \]
\[ T = \text{design spread (ft)} \] (The design spread is calculated assuming that the gutter flow can spread across the full shoulder width on bridges with not more than 2 traffic lanes. For bridges with more than 2 traffic lanes, the gutter flow can spread across the full shoulder width and half of the adjacent traffic lane width.)
\[ V = \text{gutter flow velocity (ft/sec)} \]
\[ V = \frac{1.12}{n} \times S_x^{0.67} \times S_{x}^{0.5} \times T^{-0.67} \]
\[ V_o = \text{Velocity of flow at which splash-over first occurs over a grate (ft/s.)} \]
Type 1 Drain
\[ V_0 = 10^{0.50299 + \log \left( L_d \right)} \times 0.759359 \]
Type 2 Drain
\[ V_0 = 10^{0.654078 + \log \left( L_d \right)} \times 0.474699 \]
\[ W_p = \text{width of pavement contributing to runoff (ft)} \] equals the width of deck from the face of curb to high point of crown.
\[ W_D = \text{width of deck drain at right angle to gutter flow (ft)} \]
- Type 1 Drain = 0.448 ft
- Type 2 Drain = varies
\[ X = \text{distance from the PC of a vertical curve to the inlet (ft)} \]
\[ X_T = \text{distance from PC of a vertical curve to the turning point (ft)} \]
\[ X_T = \frac{-G1 \times L}{\left( G2 - G1 \right)} \]
Figure 1

RAINFALL INTENSITY ZONES

ZONE 1

ZONE 2
ZONE 2 – INSET 2
DESIGN PROCEDURE FOR CONSTANT GRADE BRIDGES
If the bridge grade is less than 0.3 percent, then the procedure for flat bridges should be followed.

1. Determine the rainfall intensity.
   - Calculate the intensity using the Rational Method
   - Calculate the intensity using the Avoidance of Hydroplaning Method
   - Intensity due to Vision Impairment is 4.00 in/hr.
   - Use the Rational Method value for the design intensity
   - Compare the Rational Method value to the Avoidance of Hydroplaning and Vision Impairment values

2. Calculate the full gutter flow, Qf, at the design spread, T.

3. Calculate the distance to the first inlet, L0, from the upslope end of the bridge.

4. Determine if inlets are needed on the bridge.
   - If L0 is greater than the total bridge length, no inlets are required.
   - If L0 is less than the total bridge length, calculate the spacing between inlets, Lc.

5. Space inlets at Lc until the end of the bridge is reached.

6. Design the bridge end treatment.
   Inlets beyond the end of the bridge should be sized for a flow. Energy dissipation devices should be provided, if necessary, at the toe to prevent erosive velocities.
   - Where debris is a problem, assume a percentage of blockage of the drains.
   - Assume a duration time and design spread width.
   - Calculate the intensity using the Rational Method.
   - Compute tc and compare to the assumed duration time. Iterate until the assumed and computed values are equal.
   - Compute the design flow.
   - Compute the spread.
   - Iterate until the assumed duration and spread values are equal to the computed values.

DESIGN PROCEDURE FOR FLAT GRADE BRIDGES
Bridges with vertical curves are nearly flat at their low or high points. For bridges with grades less than 0.3 percent at the high or low points of vertical curves or on a constant grade, check the required inlet spacing assuming the bridge is flat. Use the flat spacing if it is less than the spacing required for a constant grade.

1. Determine the rainfall intensity using the Rational Method.
   - Use a duration of 5 minutes
   - Determine the Rainfall Intensity Zone for the bridge site and compute the intensity

2. Calculate the constant inlet spacing, Lc.

3. Determine if inlets are needed on the bridge.
   - If Lc is greater than the total bridge length, no inlets are required.

4. If inlets are needed, calculate the required inlet perimeter, P.

5. Design the bridge end treatment.

DESIGN PROCEDURE FOR VERTICAL CURVE BRIDGES
1. Compute the distance from the turning point of the vertical curve to each end of the bridge.

2. Determine the rainfall intensity using the Rational Method.
   - Assume a duration and compute the intensity.
   - Compute the time of concentration, tc, by summing the gutter and overland flow times, t0 and tg.
   - Iterate until tc equals the assumed duration.

3. Determine the spacing to the first inlet.
   - Assume a distance from the turning point to the first inlet and compute the slope at that point.
   - Compute the gutter flow, Qf, at the design spread.
   - Compute the distance to the first inlet, L0, letting K=1.
   - Iterate the process until the trial and computed values are equal.
   - If L0 is greater than the distance from the turning point to the end of bridge, than inlets are not required.

4. Determine the spacing to the next inlet, if required.
   - Select a trial spacing
   - Compute the local slope, S.
   - Calculate the gutter flow, Qf.
5. Compute the inlet efficiency, E.
6. Compute the interception coefficient, K.
7. Compute the inlet spacing and compare to the trial value.
8. Iterate the process until the trial and computed values are equal.

5. Repeat step 4 until the sum of the inlet spacing is greater than the distance from the turning point to the end of bridge.
6. Repeat steps 3, 4, & 5 for the drains on the other side of the turning point.
EXAMPLE 1 - TANGENT GRADE BRIDGE WITH NO INLETS REQUIRED

EXCEL or MATHCAD programs can be used to complete the design process. Drains on tangent grade.xls or .mcd is on the X: drive in the LRFD Design Aids, Section 2 folder.

Given:
- 500 ft bridge
- \( W_p = 18 \) ft
- \( T = 10 \) ft
- \( C = 0.9 \)
- Zone 2
- \( d = 0.006 \) ft

Step 1. Calculate the Design Rainfall Intensity

a. Rational Method

Assume duration of 44 minutes

\[
i = 10^{(1.32076-0.60121\times\log(duration))} = 10^{(1.32076-0.60121\times\log(44))} = 1.39 \text{ in/hr}
\]

\[
t_0 = 0.93 \times \left( \frac{W_p \times n}{(C \times i)^{0.4} \times S^{0.3}} \right)^{0.6} = 0.93 \times \left( \frac{18 \times 0.016}{(0.9 \times 1.39)^{0.4} \times 0.03^{0.3}} \right)^{0.6} = 1.15 \text{ minutes}
\]

\[
t_g = 484 \times \frac{S \times T^2}{C \times i \times W_p} = 484 \times \frac{0.02 \times 10^2}{0.9 \times 1.39 \times 18} = 42.95 \text{ min}
\]

\[
t_e = t_0 + t_g = 1.15 + 42.95 = 44.10 \text{ min}
\]

The assumed time of 44 minutes is approx. equal to the computed time of 44.10 minutes. Therefore, use \( i = 1.39 \) in/hr.

b. Avoidance of Hydroplaning

\[
i = \frac{64904.4}{C \times n} \left( \frac{S}{(S^2 + S^2)^{0.25}} \right) \left[ \frac{d^{1.67}}{(W_p - T)} \right] = 64904.4 \times \frac{0.02 \times 0.03^{0.3}}{(0.9 \times 1.39 \times 18)^{0.25} \times (18 - 10)^{1.67}} = 11.56 \text{ in/hr}
\]

c. Avoidance of Vision Impairment

\( i = 4.00 \) in/hr

d. Design rainfall intensity = 1.39 in/hr

Step 2. Compute full gutter flow based on the design spread of 10'.

\[
Q_f = \frac{0.56}{n} \times \frac{S_x}{n} \times 0.57 \times T^{2.67} = \frac{0.56}{0.016} \times 0.03^{1.67} \times 0.03^{0.3} \times 10^{2.67} = 4.12 \text{ cfs}
\]

Step 3. Starting at the upslope end of the bridge, compute the distance to the first inlet.

\[
L_0 = \frac{43560 \times Q_f}{C \times i \times W_p} = \frac{43560 \times 4.12}{0.9 \times 1.39 \times 18} = 7969.94 \text{ ft}
\]

Step 4. Since \( L_0 \) is greater than the total bridge length (500 ft), no inlets are required.

Step 5. Design the bridge end treatment.

- Since no drains are required, the assumed blockage = 0%
- Assumed duration = 4.5 minutes     Assumed spread = 6 feet

\[
i = 10^{(1.32076-0.60121\times\log(duration))} = 10^{(1.32076-0.60121\times\log(4.5))} = 5.49 \text{ in/hr}
\]

\[
t_0 = 0.93 \times \left( \frac{W_p \times n}{(C \times i)^{0.4} \times S^{0.3}} \right)^{0.6} = 0.93 \times \left( \frac{18 \times 0.016}{(0.9 \times 5.49)^{0.4} \times 0.03^{0.3}} \right)^{0.6} = 0.67 \text{ min}
\]

\[
t_g = 484 \times \frac{S \times T^2}{C \times i \times W_p} = 484 \times \frac{0.02 \times 6^2}{0.9 \times 5.49 \times 18} = 3.92 \text{ min}
\]

\[
t_e = t_0 + t_g = 0.67 + 3.92 = 4.59 \text{ min}
\]

4.59 minutes are approx. equal to the assumed value of 4.5 minutes.

- Compute the design flow with the percent blockage

\[
Q = C \times i \times A - (\text{blockage}) \times E \times (\text{number of drains}) \times V
\]

\[
A = \frac{W_p \times (\text{length of bridge})}{43560} = \frac{18 \times 500}{43560} = 0.207 \text{ acres}
\]

\[
Q = 0.9 \times 5.49 \times 0.207 - [0] = 1.02 \text{ cfs}
\]
• Check the design spread with the new computed flow. Compute the spread by solving the gutter flow equation for T.

\[
Q_f = \frac{0.56}{n} S_{1.67}^2 S_{0.5} T^{2.67}
\]

\[
T = \left( \frac{Q_f \times n}{0.56 \times S_{1.67}^2 S_{0.5}} \right)^{\frac{1}{2.67}} = \left( \frac{1.02 \times 0.016}{0.56 \times 0.02^{1.67} \times 0.03^{0.5}} \right)^{\frac{1}{2.67}} = 5.93 \text{ ft}
\]

This is approximately equal to the assumed value of 6 feet.

• Select an inlet that will handle 1.02 cfs and provide a pipe or paved ditch to convey the design flow from the drain to the toe of the embankment. Provide energy dissipation, if necessary, at the toe to achieve nonerosive velocities.
EXAMPLE 2 - TANGENT GRADE BRIDGE WITH INLETS REQUIRED

EXCEL or MATHCAD programs can be used to complete the design process. Drains on tangent grade.xls or .mcd is on the X: drive in the LRFD Design Aids, Section 2 folder.

Given: 2000 ft bridge S = 0.01 S_x = 0.02 W_p = 34 ft T = 10 ft n = 0.016 C = 0.9 Zone 2 d = 0.006 ft W_D = 1.0 ft L_D = 1.50 ft

Step 1. Calculate the Design Rainfall Intensity

• Rational Method

Assume duration of 12.25 minutes

\[ i = 10^{1.132076 - 0.60121 \times \log(duratio) n)} = 10^{1.132076 - 0.60121 \times \log(12.25)} = 3.00 \text{ in/hr} \]

\[ t_0 = 0.93 \times \frac{(W_p \times n)^{0.6}}{(C \times i)^{0.4} \times S^{0.3}} = 0.93 \times \frac{(34 \times 0.016)^{0.6}}{(0.9 \times 3.00)^{0.4} \times 0.9^{0.3}} = 1.73 \text{ minutes} \]

\[ t_g = 484 \times \frac{S_x \times T^2}{C \times i \times W_p} = 484 \times \frac{0.02 \times 10^2}{0.9 \times 3. \times 34} = 10.53 \text{ min} \]

\[ t_c = t_0 + t_g = 1.73 + 10.53 = 12.25 \text{ min} \]

The assumed time of 12.25 minutes is equal to the computed time of 12.25 minutes. Therefore, use \( i = 3.00 \text{ in/hr} \).

• Avoidance of Hydroplaning

\[ i = \left[ \frac{64904.4}{C \times n} \right] \frac{S_x}{(S_x + S_C)^{0.25}} \left[ \frac{g^{1.67}}{(W_p \times T)} \right] = \frac{64904.4}{0.9 \times 0.016} \times \frac{0.02}{0.02^2 + 0.01^2}^{0.25} \times 0.006^{1.67} \times (34 - 10) = 4.89 \text{ in/hr} \]

• Avoidance of Vision Impairment

\( i = 4.00 \text{ in/hr} \)

• Design rainfall intensity = 3.00 \text{ in/hr}

Step 2. Compute full gutter flow based on the design spread of 10'.

\[ Q_f = \frac{0.56}{n} \times S_x^{0.5} \times T^{2.67} \times 0.56 \times 0.9^{0.5} \times 1^{0.5} \times 10^{2.67} = 2.38 \text{ cfs} \]

Step 3. Starting at the upslope end of the bridge, compute the distance to the first inlet.

\[ L_0 = \frac{43560 \times Q_f}{C \times i \times W_p} = \frac{43560 \times 2.38}{0.9 \times 3.00 \times 34} = 1127.92 \text{ ft} \]

Step 4. Since \( L_0 \) is less than the total bridge length (2000 ft), inlets are required.

Step 5. Space inlets at \( L_c \) until the end of the bridge is reached.

• Compute the frontal flow ratio, \( E_0 \).

\[ E_0 = 1 - \left[ 1 - \frac{W_p}{T} \right]^{2.67} = 1 - \left( 1 - \frac{1.0}{10} \right)^{2.67} = 0.245 \]

• Compute the gutter velocity.

\[ V = \frac{1.12}{n} \times S_x^{0.67} \times S_C^{0.5} \times T^{2.67} = \frac{1.12}{0.016} \times 0.02^{0.67} \times 0.01^{0.5} \times 10^{2.67} = 2.38 \text{ fps} \]

• Compute the splash-over velocity.

\[ V_0 = 10^{0.654078 \times \log( L_c) - 0.474699} = 10^{0.654078 \times \log(1.50) - 0.474699} = 3.89 \text{ fps} \]

• Compute the frontal flow intercept efficiency \( R_f = 1 - 0.09 \times (V - V_0) = 1 - 0.09 \times (2.38 - 3.89) = 1.136 \)

Maximum \( R_f = 1.0 \)

• Compute the inlet efficiency \( E = R_f \times E_0 = 1.0 \times 0.245 = 0.245 \)

• Compute the constant spacing between the remainder of the inlets.

\[ L_c = \frac{43560 \times Q_f \times E}{C \times i \times W_p} = \frac{43560 \times 2.38 \times 0.245}{0.9 \times 3.00 \times 34} = 276.57 \text{ ft} \]
• Compute the number of drains per side
\[
\text{number} = \frac{(\text{bridge length}) - L_0}{L_c} = \frac{2000 - 1129.33}{276.57} = 3 \text{ drains}
\]

Step 6. Design the bridge end treatment.

• Since drains are required, assume blockage = 50%
• Assumed duration = 15.5 minutes  
  Assumed spread = 10.6 feet

\[
i = 10^{1.132076 - 0.60121 \log(\text{duration})} = 10^{1.132076 - 0.60121 \log(15.5)} = 2.61 \text{ in} / \text{hr}
\]

\[
t_0 = 0.93 * \left(\frac{W_p * n}{C * i * S}\right)^{0.6} = 0.93 * \frac{(34 * 0.016)^{0.6}}{(0.9 * 2.61)^{0.4} * 0.01^{0.3}} = 1.83 \text{ min}
\]

\[
t_g = 484 \frac{S_x \cdot T^2}{C \cdot i \cdot W_p} = 484 \frac{0.02 \cdot 10.6^2}{0.9 * 2.61 * 34} = 13.62 \text{ min}
\]

\[
t_e = t_0 + t_g = 1.83 + 13.62 = 15.45 \text{ min}
\]
15.45 minutes are approx. equal to the assumed value of 15.5 minutes.

• Compute the design flow with the percent blockage

\[
Q = C * i * A - [(\text{blockage}) * E * (\text{number of drains}) * V]
\]

\[
A = \frac{W_p * (\text{length of bridge})}{43560} = \frac{34 \cdot 2000}{43560} = 1.56 \text{ acres}
\]

\[
Q = 0.9 * 3.21 * 1.56 - [0.50 * 0.245 * 3 * 2.38] = 2.79 \text{ cfs}
\]

Check the design spread with the new computed flow. Compute the spread by solving the gutter flow equation for T.

\[
Q_f = \frac{0.56}{n} * S_x^{1.67} * S^{0.5} * T^{2.67}
\]

\[
T = \left(\frac{Q_f * n}{0.56 * S_x^{1.67} * S^{0.5}}\right)^{1/2.67} = \left(\frac{2.79 * 0.016}{0.56 * 0.02^{1.67} * 0.01^{0.5}}\right)^{1/2.67} = 10.61 \text{ ft}
\]

This is equal to the assumed value of 10.6 feet.

• Select an inlet that will handle 2.79 cfs and provide a pipe or paved ditch to convey the design flow from the drain to the toe of the embankment. Provide energy dissipation, if necessary, at the toe to achieve nonerosive velocities.
EXAMPLE 3 – FLAT GRADE BRIDGE
EXCEL or MATHCAD programs can be used to complete the design process. Drains on flat grade(.xls or .mcd) is on the X: drive in the LRFD Design Aids, Section 2 folder.

Given:

- 4000 ft bridge
- T = 10 ft
- S<sub>t</sub> = 0.02
- S = 0
- n = 0.016
- C = 0.9
- W<sub>p</sub> = 34 ft
- Rainfall Zone = 2
- t<sub>c</sub> = 5 minutes

Step 1. Determine the rainfall intensity.
- Rational Method
  
  \[ i = 10^{1.132076-0.6012 \log(duration)} = 10^{1.132076-0.6012 \log(5)} = 5.15 \text{ in/hr} \]

- Avoidance of Hydroplaning
  
  \[ i = \left[ \frac{64904.4}{C \cdot n} \right] \left[ \frac{S_x}{(S_x^2 + S^2)^{0.25}} \right] \left[ \frac{d^{1.67}}{W_p \cdot T} \right] \]

  \[ = \frac{64904.4}{0.9 \cdot 0.016} \cdot \frac{0.02}{(0.02^2 + 0)^{0.25}} \cdot \frac{0.006^{1.67}}{(34 - 10)} = 5.17 \text{ in/hr} \]

- Driver Vision Impairment \( i = 4.00 \text{ in/hr} \)
- Design intensity = 5.15 in/hr

Step 2. Compute the inlet spacing

\[ L_c = \frac{1312 \cdot S_x^{1.44} \cdot T^{2.11}}{(n \cdot C \cdot i \cdot W_p^{0.67})} = \frac{1312 \cdot 0.02^{1.44} \cdot 10^{2.11}}{(0.016 \cdot 0.9 \cdot 5.15 \cdot 34)^{0.67}} = 325.32 \text{ ft} \]

Step 3. Since \( L_c \) is less than the bridge length (4000 ft), inlets are needed.

Step 4. Compute the total inlet perimeter

\[ P = \frac{(C \cdot i \cdot W_p)^{0.33} \cdot T^{0.61}}{102.5 \cdot S_x^{0.06} \cdot n^{0.67}} = \frac{(0.9 \cdot 5.15 \cdot 34)^{0.33} \cdot 10^{0.61}}{102.5 \cdot 0.02^{0.06} \cdot 0.016^{0.67}} = 4.26 \text{ ft} \]

If the inlet is adjacent to the curb, then the sum of the other 3 sides should equal the computed inlet perimeter. A curb opening should equal the computed inlet perimeter.

Solutions.

- Number of inlets required on each side of the deck.

  \[ Number \ of \ drains = \frac{L_b - L_c}{L_c} = 11.3 \quad \text{Use } N_D = 12 \]

  \[ Drain \ Spacing = \frac{L_b}{N_D + 1} = 307.69 \text{ ft} \]

- Type 2 Drain
  12 drains spaced at 307.69 feet along the length of bridge.
  
  \[ Width \ of \ drain = \frac{4.26 - 0.79}{2} = 1.74 \text{ ft} \]

- Type 1 Drain
  12 locations spaced at 307.69 feet along the length of bridge.
  
  \[ P_1 = 2 \cdot 0.448 + 0.78 = 1.676 \]
  
  Number of drains required at each location = \[ \frac{4.26}{1.676} = 2.6 \quad \text{Use 3} \]
EXAMPLE 4 – VERTICAL CURVE BRIDGE

EXCEL or MATHCAD programs can be used to complete the design process. Drains on vertical curve.xls or .mcd is on the X: drive in the LRFD Design Aids, Section 2 folder.

Given: 3000 ft bridge
n = 0.016  Sx = 0.02  T = 10 ft

d = 0.006 ft  C = 0.9  Wp = 34 ft

Zone 2  W = 34 ft  L = 1.0 ft

Zone 2  G1 = 0.01  G2 = -0.01

Length of vertical curve = 3000 ft
PC station = 10+00  Begin bridge station = 10+00

Step 1. Compute the distance from the turning point of the vertical curve to each end of the bridge.
- Locate the distance of the turning point from the PC.
- Compute the distance from the turning point to each end of the bridge.

\[
X = \frac{-G1 \cdot L}{(G2 - G1)} = \frac{-0.01 \cdot 3000}{(-0.01 - 0.01)} = 1500 \text{ ft}
\]

Step 2. Determine the rainfall intensity (turning point to end of bridge)
- Rational Method  S = absolute value (G2) = 0.01
  Assume duration of 12.25 minutes
  \[
i = 10 \frac{1.132076 - 0.60121 \log \text{duration}}{1.132076 - 0.60121 \log (12.25)} = 3.01 \text{ in/hr}
\]

\[
t_g = \frac{484 \cdot S \cdot T^2}{C \cdot i \cdot W_p} = \frac{484 \cdot 0.02 \cdot 10^2}{0.9 \cdot 3.01 \cdot 34} = 10.53 \text{ min}
\]

\[
t_0 = 0.93 \cdot \frac{(W_p \cdot n)^{0.6}}{(C + i)^{0.4} \cdot S^{0.3}} = 0.93 \cdot \frac{(34 \cdot 0.016)^{0.6}}{(0.9 \cdot 3.01)^{0.4} \cdot 0.01^{0.3}} = 1.73 \text{ min}
\]

\[
t_c = t_0 + t_g = 1.73 + 10.53 = 12.25 \text{ min}
\]

The assumed time of 12.25 minutes is equal to the computed time. Therefore, use i = 3.01 in/hr.

- Avoidance of Hydroplaning
  \[
i = \left[ \frac{64904.4}{C \cdot n} \right] \cdot \left[ \frac{S}{(S_x + S^2)^{0.25}} \right] \cdot \left[ \frac{d^{1.67}}{(W_p - T)} \right] = \left( \frac{64904.4}{0.9 \cdot 0.016} \right) \cdot \frac{0.02}{(0.02^2 + 0.01)}^{0.25} \cdot \frac{0.006^{1.67}}{(34 - 10)} = 4.89 \text{ in/hr}
\]

- Avoidance of Vision Impairment
  i = 4.00 in/hr

- Design rainfall intensity = 3.01 in/hr

Step 3. Assume the distance to the first inlet from the turning point to the end of bridge = 848 ft
Distance from PC to first inlet, X = 848 + 1500 = 2348 ft.
- Compute the slope at the assumed first inlet location
  \[
  S = G1 + \frac{(G2 - G1) \cdot X}{L_{vc}} = 0.01 + \frac{(-0.01 - 0.01) \cdot 2348}{3000} = -0.00565
  \]
  Use the absolute value of S. Therefore, S = 0.00565 ft/ft

Step 4. Compute the gutter flow at the design spread
  \[
  Q_f = \frac{0.56}{n} \cdot S_x^{1.67} \cdot S^{0.5} \cdot T^{2.67} = \frac{0.56}{0.016} \cdot 0.02^{1.67} \cdot 0.00565^{0.5} \cdot 10^{2.67} = 1.79 \text{ cfs}
  \]

Step 5. Compute the distance to the first inlet and compare to the assumed value.
K = 1.0 for the first inlet

\[
L_c = \frac{43560 \cdot Q_f \cdot K}{C \cdot i \cdot W_p} = \frac{43560 \cdot 1.79 \cdot 1.0}{0.9 \cdot 3.01 \cdot 34} = 848.07 \text{ ft}
\]

Assumed value of 848 ft equals computed value of 848.07 ft.
Inlets are required since the distance to the first inlet, 848 ft, is less than the distance from the turning point to the end of bridge, 1500 ft.
Step 6. Determine the spacing to the next inlet.

- Assume a distance to the next inlet and compute the slope at that point
  Assume 381 ft to the next drain
  \[ X = 1500 + 848 + 381 = 2729 \text{ ft from the PC} \]
  \[ S = G1 + \frac{(G2 - G1) \times X}{L_{vc}} = 0.01 + \frac{(-0.01 - 0.01) \times 2729}{3000} = -0.00819 \]
  
  Use the absolute value of S. Therefore, \( S = 0.00819 \).

- Compute the gutter flow at the design spread
  \[ Q_f = \frac{0.56}{n} \times S_{x}^{0.67} \times S^{0.5} \times T^{2.67} = \frac{0.56}{0.016} \times \frac{0.02167}{0.00819^{0.5} \times 10^{-2.67}} = 2.16 \text{ cfs} \]

- Compute the inlet efficiency, \( E \).
  \[ E_0 = 1 - \left( 1 - \frac{W_d}{T} \right)^{2.67} = 1 - \left( 1 - \frac{10}{10} \right)^{1.67} = 0.245 \]

- Compute the interception coefficient, \( K \).
  \[ K = 1 - \left( 1 - E \right) \left( \frac{S_u}{S} \right)^{0.5} = 1 - \left( 1 - 0.245 \right) \left( \frac{0.00565}{0.00819} \right)^{0.5} = 0.373 \]

- Compute the inlet spacing and compare to the assumed value.
  \[ L_i = \frac{43560 \times Q_f \times K}{C \times i \times W_p} = \frac{43560 \times 2.16 \times 0.373}{0.9 \times 3.01 \times 34} = 380.72 \text{ ft} \]

  The computed length of 380.72 ft equals the assumed value of 381 ft.

Step 7. Repeat step 6 for each inlet until the end of the bridge is reached.

Since the bridge is this example is symmetrical, the drain configuration will be the same from the turning point to both the begin/end of bridge. The following table summarizes the inlet spacing.

<table>
<thead>
<tr>
<th>Inlet</th>
<th>Spacing, Li Feet</th>
<th>Station</th>
<th>Slope, S Ft/ft</th>
<th>Gutter flow, Q cfs</th>
</tr>
</thead>
<tbody>
<tr>
<td>1</td>
<td>848.07</td>
<td>3348.07</td>
<td>0.00565</td>
<td>1.79</td>
</tr>
<tr>
<td>2</td>
<td>380.72</td>
<td>3728.79</td>
<td>0.00819</td>
<td>2.16</td>
</tr>
<tr>
<td>3</td>
<td>357.40</td>
<td></td>
<td></td>
<td></td>
</tr>
</tbody>
</table>

Only 2 drains are required on the bridge. The 357.40 ft to the third drain puts it at station 4086.19, which is beyond the end of the bridge.

Step 8. Design bridge end treatment

- Since drains are required, assume blockage = 50%
- Assumed duration = 23.5 minutes  Assumed spread = 11.75 feet
\[ i = 10^{1.132076 - 0.60121 \log(dur)} = 10^{1.132076 - 0.60121 \log 23.5} = 2.03 \text{ in/hr} \]

\[ t_0 = 0.93 \frac{(W_p \times n)^{0.6}}{(C \times i)^{0.4} \times S^{0.3}} = 0.93 \frac{(34 \times 0.016)^{0.6}}{(0.9 \times 2.03)^{0.4} \times 0.01^{0.3}} = 2.02 \text{ min} \]

\[ t_g = \frac{484 \times S_x \times T^2}{C \times i \times W_p} = \frac{484 \times 0.02 \times 11.75^2}{0.9 \times 2.03 \times 34} = 21.50 \text{ min} \]

\[ t_c = t_0 + t_g = 2.02 + 21.50 = 23.52 \text{ min} \]

23.5 minutes are equal to the assumed value.

• Compute the design flow with the percent blockage

\[ Q = C \times i \times A - ((\text{blockage}) \times E \times (\text{number of drains}) \times V) \]

\[ A = \frac{W_p \times (\text{length of bridge})}{43560} = \frac{34 \times 3000}{43560} = 2.34 \text{ acres} \]

\[ V = \frac{1.12}{n} \times S_x^{0.67} \times S^{0.5} \times T^{0.67} = \frac{1.12}{0.016} \times 0.02^{0.67} \times 0.01^{0.5} \times 11.75^{0.67} = 2.653 \text{ fps} \]

\[ Q = 0.9 \times 2.03 \times 2.34 - (0.50 \times 0.245 \times 2 \times 2.653) = 3.63 \text{ cfs} \]

• Check the design spread with the new computed flow. Compute the spread by solving the gutter flow equation for T.

\[ Q_f = \frac{0.56}{n} \times S_x^{1.67} \times S^{0.5} \times T^{2.67} \]

\[ T = \left( \frac{Q_f \times n}{0.56 \times S_x^{1.67} \times S^{0.5}} \right)^{2.67} = \left( \frac{3.63 \times 0.016}{0.56 \times 0.02^{1.67} \times 0.01^{0.5}} \right)^{2.67} = 11.71 \text{ ft} \]

This is equal to the assumed value of 11.75 feet.

• Select an inlet that will handle 3.63 cfs and provide a pipe or paved ditch to convey the design flow from the drain to the toe of the embankment. Provide energy dissipation, if necessary, at the toe to achieve nonerosive velocities.